Residual Static Strength of Tubular T-Joints with Fatigue Surface Cracks

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Abstract: To study the influence of surface fatigue cracks on the static strength of circular tubular T-joints, experimental tests on four full-scale circular tubular T-joints are carried out. Firstly, surface fatigue cracks are prefabricated by fatigue test in three circular tubular T-joints. Thereafter, experimental tests on the static strength of all the circular tubular T-joints with and without fatigue cracks under axial tension are carried out. Through the comparative analysis, the influences of the fatigue crack on the failure mode and the static strength of circular T-joints are concluded.

Keywords: Experimental study, fatigue surface cracks, residual static strength, tubular T-joint.

1. INTRODUCTION

Tubular joints have been widely used in offshore structures, bridge structures, space structures and so on, due to their beautiful configurations, excellent mechanical properties and high strength-weight ratio. However, the service environment of these structures is very bad and special, so they often suffer from the cyclic loadings (such as sea wave, sea wind, earthquake and tide). Due to that there is obvious stress concentration phenomenon at the intersection position of chord and brace, the failure of tubular joint often performs that the fatigue cracks occur there. Propagating of fatigue cracks can decrease the stiffness of joint. Which may causes that the joint occurs brittle fracture at the crack position, and then it is possible for the structure to collapse ultimately. Thus, it is necessary and meaningful to study the static strength of tubular T-joint with fatigue surface cracks.

Recently, the main researches of tubular joints are mainly focused on static study, quasi-static study and fatigue property by scholars. However, the researches of residual static strength of tubular joints with fatigue surface cracks are much less. Yang [1] had done the experiment of tubular T-joints with through cracks. It is found that the static strength of cracked specimen decreases by 25% and 10% than that of uncracked specimen under tension and compression respectively, and the failure mode is brittle fracture failure. The researchers [2-7] had done some experimental study on the tubular T-joints with artificial notch cracks, and verified the reliability of different kinds of FAD curves. Due to that the cracks were notched by hand, it could not response the actual fatigue surface cracks.

In this paper, the fatigue surface cracks of tubular joints are prefabricated by fatigue test. Base on the contrastive analysis of cracked and uncracked specimens, the failure mode and static performance are clarified.

2. EXPERIMENTAL INVESTIGATION

The geometry and some normalized geometrical parameters commonly used for describing a tubular T-joint are shown in Fig. (1).

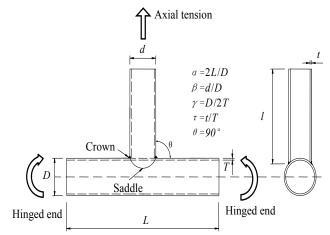


Fig. (1). Geometry of circular tubular T-joint.



Fig. (2). Test rig.

Model	Туре	L (mm)	L (mm)	D (mm)	d (mm)	T (mm)	β	γ	τ
M-A1	Cracked	2000	300	159	45	8	0.28	9.9	0.75
M-A2	Uncracked	2000	300	159	45	8	0.28	9.9	0.75
M-B1	Cracked	2000	400	180	133	8	0.739	11.25	0.75
M-B2	Uncracked	2000	400	180	133	8	0.739	11.25	0.75

Table 1. Geometric parameters of specimens.

Table 2. Material properties of steel tube.

Model	Elastic Modulus (GPa)	Yield Strength (MPa)	Ultimate Tensile Strength (MPa)	Elongation
M-A	200	318	573	23.45%
M-B	171.8	390	560	16.6%

Two groups (M-A and M-B) of specimens are fabricated, and the geometric parameters of the two groups of specimens are listed in Table 1. Each group includes two specimens (uncracked and cracked). The fatigue surface cracks of specimens are prefabricated on the fatigue test machine (as shown in Fig. 2). Afterwards, all of specimens are carried out the static strength test under axial tension. The material properties of steel tubes are tested by uni-axial tensile test (as shown in Fig. 3), and all of them are listed in Table 2.



Fig. (3). The rig of material test.

3. PREFABRICATION OF FATIGUE SURFACE CRACKS

To know where the surface crack will come into being beforehand, before fatigue test, the stress distribution around the weld toe of tubular T-joints should be tested. 8 groups of strain gages are sticked around the weld toe (as shown in Fig. 4). Each group includes 2 strain gages, and the location of strain gage is determined according to CIDECT design guide [8].

The hot spot stress distribution curves are shown in Fig. (5). The point θ° is defined at the crown position of tubular joint. It can be seen that the hot spot stress of the two specims both appear at the 180° position from Fig. (5), and it

can be concluded that the fatigue surface cracks will come into being at this position.

Sine wave load control method is adopted to apply the fatigue load for tubular T-joint. The specific loading scheme is shown in Fig. (6). The peak value load of sine wave curve is determined by the maximum hot spot stress, and the value should not exceed the yield stress. The position and detail dimensions of fatigue cracks are shown in Fig. (7). Where, '2c' is the length of cracks, and 'a' is the depth of the deepest position of cracks.

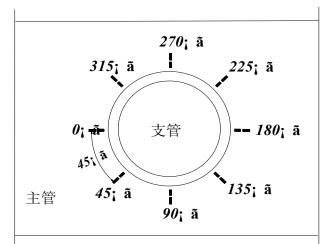




Fig. (4). Arrangement of strain gauge.

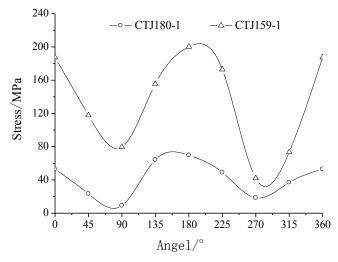


Fig. (5). Hot spot stress distribution around the weld toe of T-joints.

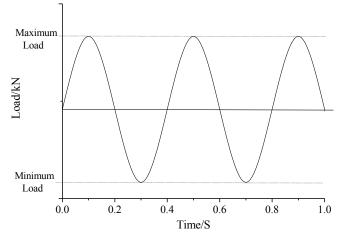


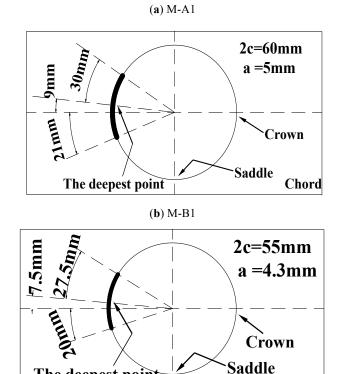
Fig. (6). Loading scheme of fatigue test.

4. STATIC STRENGTH TEST

After the fatigue test, the static strength tests of tubular T-joints are carried out. The SDS500 test rig will be used to tension the specimens which have been prefabricated the fatigue surface cracks (as shown in Fig. 8). The constraint conditions of specimens remain unchanged. In the course of test, the load control method is used in elastic stage, and the speed of loading is 5 kN/min. When the specimens come into plastic stage, the displacement control will be used, and the speed of loading is 2 mm/min.

5. ANALYSIS OF EXPERIMENTAL RESULTS

The failure modes of the two specimens without fatigue surface cracks are shown in Fig. (9). It can been seen that the chord of M-B1 specimen occurs larger bending deformation, due to that the chord of specimen with larger value of β (0.739) is of stronger resistivity in the radial direction. For M-A1 specimen, due to that the value of $\beta(0.28)$ is relatively small, the larger local plastic deformation appears on the surface of chord around the weld toe. Though the failure position and state are different, the failure modes of the two intact tubular T-joints are both overall plastic ultimate strength failure under axial tension



Chord

Fig. (7). Details of fatigue cracks.

The deepest point



Fig. (8). SDS500 test rig.

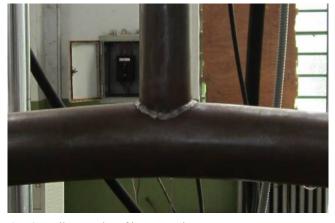
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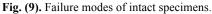
Fig. (10) shows the failure modes of specimens with fatigue surface cracks. For the M-A2 and M-B2 specimens, due to the existence of crack, the cracked side of specimen appears plastic range firstly. With the increasing of loading, another side of it also appears plastic range. When the loading reaches a critical value, the fatigue surface crack fractures suddenly, and it performs brittle failure. Thus, the stiffness of cracked side declines, which causes that the bearing capacity also declines (as shown in Fig. 8). Based on experimental observation, the failure modes of the two specimens are brittle fracture failure eventually and the cracks propagates alone the radial direction of the chord wall.

(a) M-A1



(**b**) M-B1





The load-displacement curves of specimens are shown in Fig. (12). It can been seen that the load-displacement curves of cracked specimens are coincident with that of uncracked ones before arriving 'A' point loading, which illustrates that the fatigue surface cracks have little effects on the static strength of tubular joints during this phase. When the loadings arrive 'A' point, the fatigue surface cracks propagate rapidly and suddenly, and then penetrate the chord wall. The stiffness of cracked specimen begins to decline, which causes that the bearing capacity decreases. Thus, 'A' point loading is a critical loading for the cracked specimen. The twice-elastic-slope (TES) method [9] is used to determine the ultimate strength of the specimens (as shown in Fig. 11). The bearing capacities of M-A specimens with

and without fatigue surface are 124.6 KN and 131.9 KN respectively, and that of M-B specimens are 204.9 KN and 222.3 KN respectively. From the above dates, it can be found that the bearing capacities of the two specimens have been decreased by 5.53% and 7.8% respectively. Which indicates that the bearing capacity of tubular joint can been weakened by fatigue surface cracks.

(a) M-A2



(**b**) M-B2





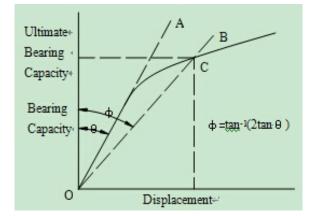


Fig. (11). TEScriteria.

6. VALIDATION OF COMPUTATION FORMULA

The plastic collapse loads of tubular T-joint with surface cracks can be obtained by multiplying the strength of

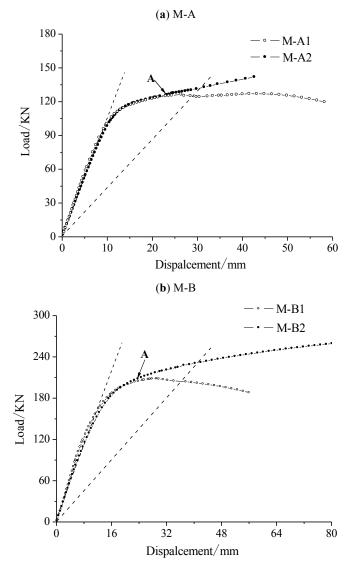


Fig. (12). Load-displacement curves of the specimens.

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equivalent uncracked joint by an 'area reduction factor'(ARF) [10]. The computation formula of ARF is as follows:

$$F_{AR} = \left(1 - \frac{A_{crack}}{T \times weld \ length}\right) \left(\frac{1}{Q_{\beta}}\right) \tag{1}$$

where, A_{crack} is the area of crack surface, T is the thickness of chord wall, and Q_{β} is a coefficient related to β , as shown in formula (2).

$$\begin{cases} Q_{\beta} = 1.0 & (\beta \le 0.6) \\ Q_{\beta} = \frac{0.3}{\beta(1 - 0.833\beta)} & (\beta > 0.6) \end{cases}$$
(2)

The weld length is a space curve, and its function expression is very complex. The intersecting line parameter equation is shown in formula (3).

$$\begin{cases} X = R \cos\left(\arcsin\left(\frac{r}{R}\sin\alpha\right)\right) \\ Y = r \sin\alpha \\ Z = r \cos\alpha \end{cases}$$
(3)

The above formula is calculated by curvilinear integral, and the calculation formula of intersecting line's length is obtained. Due to that the effect of weld size is very great, the weld size should be considered as an important factor to calculate the length of intersecting line. The calculation formula of intersecting line's length is shown in formula (4).

$$L = \int_{0}^{2\pi} \sqrt{\frac{(r+t_{w})^{4} \sin^{2} \alpha \cos^{2} \alpha}{R^{2} - (r+t_{w})^{2} \sin^{2} \alpha}} + (r+t_{w})^{2} d\alpha$$
(4)

where, r is the radius of brace, R is the radius of chord, tw is the weld size, α is the angel of brace and chord, L is the length of intersecting line.

Based on the above formula and the measured value of geometric dimension of specimens, the values of every parameter are calculated and listed in Table **3**.

Table 3. The results of parameters.

Model	L (mm)	$A_{crack}(m^2)$	T (mm)	ARF
M-A	213.03	175.71	8	0.8969
M-B	515.98	184.63	8	0.9012

The values of ARF are calculated by the above formula (As shown in Table 4), and the bearing capacities are also all listed in Table 4. Where, F_u is the bearing capacity of uncracked specimens; F_f is the bearing capacity of cracked specimens calculated by 'ARF' method; F_c is the bearing capacity of cracked specimens; P is the percentage of the difference between testing and calculating by formula for tubular T-joint with surface cracks.

 Table 4.
 The results of testing and computation.

Model	$F_u(KN)$	F _f (KN)	$F_c(KN)$	Р
M-A	131.9	118.3	124.6	5.3%
M-B	222.3	200.07	204.9	2.5%

From datum of Table **4**, it can be found that the percentages of the difference between testing and calculating by formula for the two tubular T-joints with surface cracks are both less than 6%, and it is in an acceptable range. Thus,

it is reliable and acceptable that the computing formula of 'ARF' method is used to calculate the static strength of tubular T-joints with fatigue surface cracks.

CONCLUSION

Based on the analysis of tubular T-joint with fatigue surface crack under axial tension by experimental investigate method, the following conclusions can be drawn.

- 1) The failure mode of uncracked tubular T-joint is overall plastic ultimate strength failure. However, the failure mode of tubular T-joint with fatigue surface crack is brittle fracture failure eventually, and the crack propagates alone the radial direction of the chord wall.
- 2) The stiffness of the cracked tubular joint is not influenced by the fatigue crack greatly in linear and elastic stage.
- 3) The residual strengths of the cracked tubular T-joints decrease by 5.53% and 7.8% respectively compared with corresponding uncracked tubular T-joint. It indicates that the bearing capacity of tubular joint can been weakened by fatigue surface cracks.
- 4) It is reliable and acceptable to calculate the static strength of tubular T-joints with fatigue surface cracks by the computing formula of 'ARF' method.

CONFLICT OF INTEREST

The authors confirm that this article content has no conflict of interest.

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