

Experimental Analyses of Mechanical Performance of CFST Column to Assembled Steel H-Beam Connections

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Abstract: The mechanical performance of a new proposed type of cross-shaped connection with concrete-filled steel tubular (CFST) column and assembled steel H-beam was investigated. Cyclic loading tests on the cross-shaped connections are carried out by using MTS servo loading system. Comparative analyses are conducted based on the experimental results including hysteretic curves, skeleton curves and stiffness degradation curves as well as ductility coefficients of cyclic loading tests. Furthermore, effects of geometric parameters of ring-stiffened plate, axial compression ratio and backing plate on the deformation performance of cross-shaped connections are analyzed. The results show that the increase of width of ring plate and the shape change of ring plate from square to circle can both significantly improve the ductility and the hysteretic characteristics of connection. It is emphasized that the specimen with square ring plates is of better deformation performance but lower bearing capacity than the ones with circular ring plates. Besides, the backing plates always have positive effects on the hysteretic characteristics, ductility and energy dissipation of the CFST column connections. Conversely, the increase of axial compression ratio contributes negatively to both the bearing capacity and deformation performance of the connection specimens. It can be concluded that the presented cross-shaped connection is of good deformation performance relating to hysteretic characteristics, energy dissipation and ductility, which can provide reference towards engineering practice with potential perspective application.

Keywords: Cross-shaped connection, cyclic loading, ductility, hysteretic behavior.

1. INTRODUCTION

In the last thirty years, concrete-filled steel tubular (CFST) columns were widely employed as compression members in high-rise building, long-span bridge and underground structures due to the advantages of good performance, construction convenience and fine fire resistance as well as lower construction cost [1, 2]. It is well-known that concrete-filled steel tubular columns increase the strength and the stiffness of the section by confining the concrete panel in the steel tube. With the application of CFST columns into engineering practices, different types of connections have been developed to connect flange beams to tube columns [3]. According to the 'Specification for design and construction of steel pipe concrete structure (CECS28:90)', beam-column connections should be of fine entirety and satisfy the criterion of strong column and weak beam. The corresponding modes of load transfer and construction technology are two key aspects of connection design, as emphasized by RICLES and GREGORY *et al.* [4, 5]. Researchers have conducted many studies on CFST column and beam connections. As an effective experimental measure, cyclic loading tests are adopted by most researchers

in analyzing deformation capacity of CFST column-beam connections. Typically, seismic performances of concrete-filled steel square tube column connections with H beams was investigated by MORINO *et al.* [6]. And the ductility and energy dissipation of concrete-filled steel square tubular column connections were discussed in [7, 8]. Furthermore, both the characteristics of stress concentration and plastic hinge in embedded type of CFST column connections were reported by BEUTEL *et al.* and Li *et al.* respectively [9, 10]. Based on previous engineering practice and research, effects of geometric features of connections, axial compression ratio and other factors on the performance of CFST column connections such as ductility and hysteretic characteristics were discussed in [11-15]. However, most presented types of CFST column connections are connected by welding technique, this may lead to construction inconvenience and inefficiency especially for the complicated frameworks and structures. Whereas, fabricated beams with connector of high strength friction grip bolt are promising for designing CFST column connections, which is of easy operation and can satisfy the strength needs as well. In addition, there was little research on CFST column connections with assembled beams without welding both in theoretical analysis and experimental investigation.

The main purposes of this work are to propose and design a new type of cross-shaped connection with CFST column and assembled steel H-beam and to carry out cyclic loading tests by using MTS servo loading system. On the basis of experimental results, comparative analyses are also

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conducted to discuss the effects of geometric parameters of ring-stiffened plate, axial compression ratio and backing plate on the deformation performance of cross-shaped connections.

2. DESIGNED CONNECTION AND EXPERIMENTAL PROGRAM

2.1. Designed Connection and Test Specimens

The details of the connections used in this research are shown in Fig. (1), in which both the three-dimensional

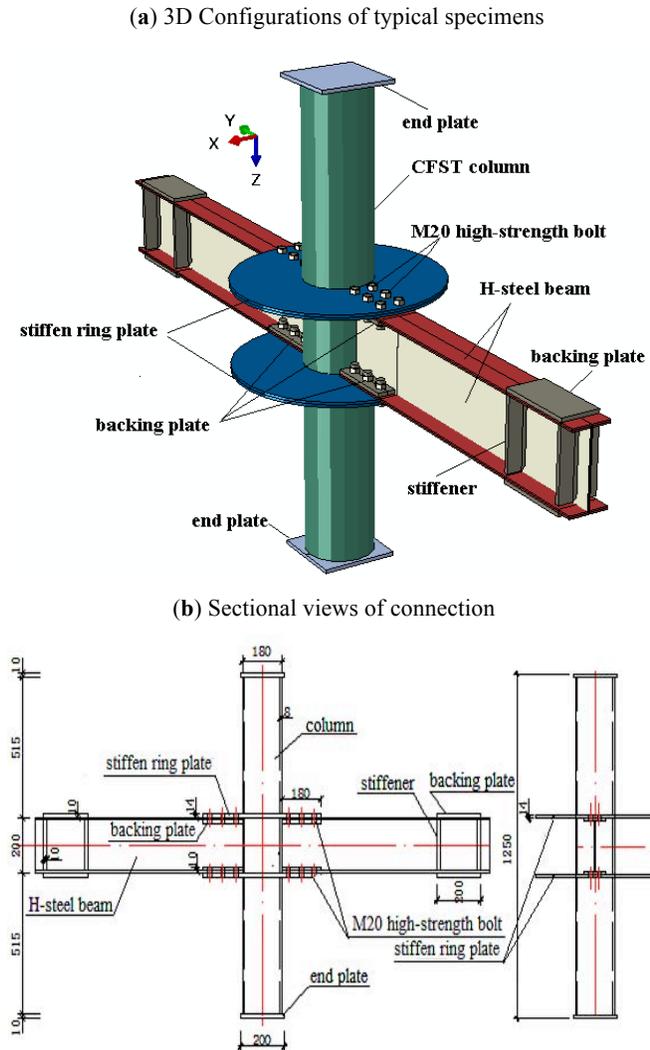


Fig. (1). Details of connection.

Table 1. Steel properties of specimens.

Specimen	Thickness (mm)	Yield Strength (MPa)	Ultimate Stress (MPa)	Elastic Modulus (MPa)	Poisson Ratio	Elongation (%)
Tube shell	8	294.6	386.7	1.9×10^5	0.29	20.6%
H-beam flange	8	310.3	432.8	2.1×10^5	0.24	22.1%
H-beam web	5.5	370.6	496.7	2.0×10^5	0.28	21.4%
Stiffen ring plate	14	272.2	410.7	1.9×10^5	0.28	24.6%
Backing plate	10	278.6	381.6	2.1×10^5	0.27	23.6%

configurations and the sectional views of connection are depicted. The types of steel used in the specimens include Q235, Q345, Q390 and Q420, and the strengths of all types are obtained by tensional coupon tests. The detailed steel properties of specimens are listed in Table 1. The designed strength grade of filled-concrete is C60 and the corresponding average value of compressive strength for 150mm cube specimen is $f_{cu}=64$ MPa. Six specimens were manufactured according to the schedule of specimens listed in Table 2. Especially, square ring plate was adopted in the specimen F12D3, of which the diagonal line coincides with the median of H-beam. All H-beams of connection specimens were attached to CFST column stubs by high strength steel bolts as cantilevers.

2.2. Test Set-Up

Cyclic loading tests on the cross-shaped connections are carried out by using MTS servo loading system. The test set-up and loading system of the connection experiments are shown in Fig. (2).

The dual actuators were controlled by computer intelligent system. Test results could be gathered by the system automatically as well. The column was assumed to be pin-supported at the end and the beam was loaded at an assumed pin at the position of actuators. The distance measured from the column centerline to the loading point of actuator was 900 mm. As shown in Fig. (3), strain gauges were applied to the specimens in order to measure strain distributions and principal strain magnitudes and directions at typical points. Fig. (4) shows the displacement transducers (DT) employed to measure the beam tip displacement and rotation contributions of different parts of the specimens.

The axial force for CFST column was of 5000kN magnitude and coincident with the central line of itself. The cyclic loading according to a quasi-static cyclic displacement pattern was defined in terms of beam end displacement and was applied vertically through the dual actuators to the H-beams based on the loading scheme adopted in Refs[16]. The loading pattern was applied to each specimen until failure occurred.

3. EXPERIMENTAL RESULTS AND DISCUSSION

3.1. Hysteretic Characteristics

The hysteretic curves of load *versus* displacement relationships obtained from the tests are depicted in Fig. (5). It is obvious that the hysteretic characteristics of each specimen become much more robust with the cyclic increase of displacements, which indicates the fine deformation

Table 2. Schedule of specimens.

Specimen	Backing Plate	Column (mm)		H-Beam (mm)		Axial Compression Ratio	Width of Ring Plate (mm)
		Cross Section	Length	Cross Section	Length		
H18D3	with	$\phi 180 \times 8.00$	1250	200×100×8×5.5	2080	0.3	180 (circular)
H18W3	without	$\phi 180 \times 8.00$	1250	200×100×8×5.5	2080	0.3	180 (circular)
H18D6	with	$\phi 180 \times 8.00$	1250	200×100×8×5.5	2080	0.6	180 (circular)
H18W6	without	$\phi 180 \times 8.00$	1250	200×100×8×5.5	2080	0.6	180 (circular)
H12D3	with	$\phi 180 \times 8.00$	1250	200×100×8×5.5	2080	0.3	120 (circular)
F12D3	with	$\phi 180 \times 8.00$	1250	200×100×8×5.5	2080	0.3	127(square)

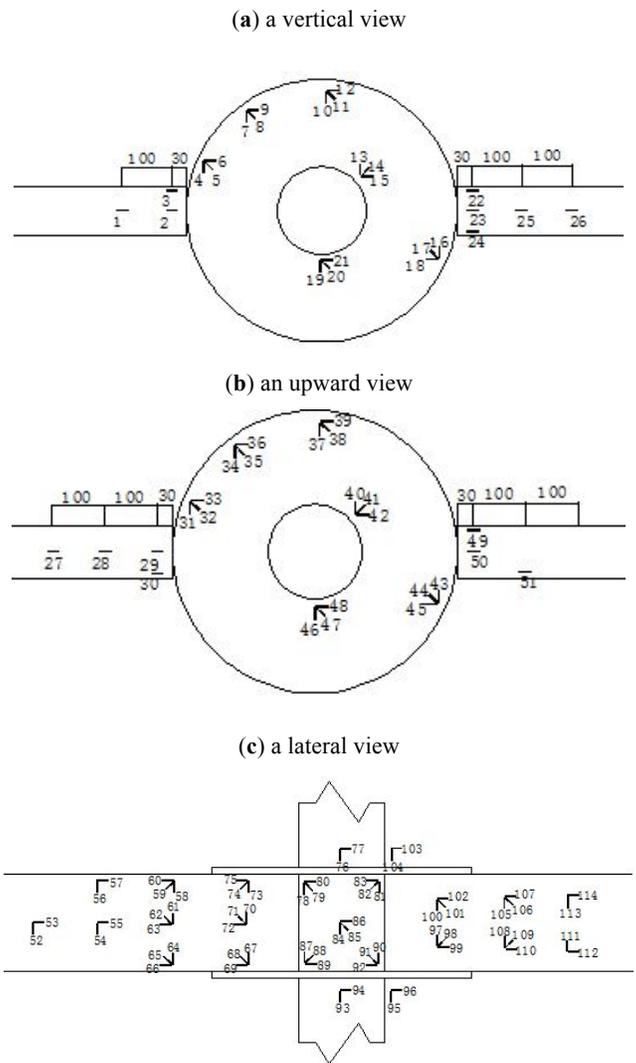
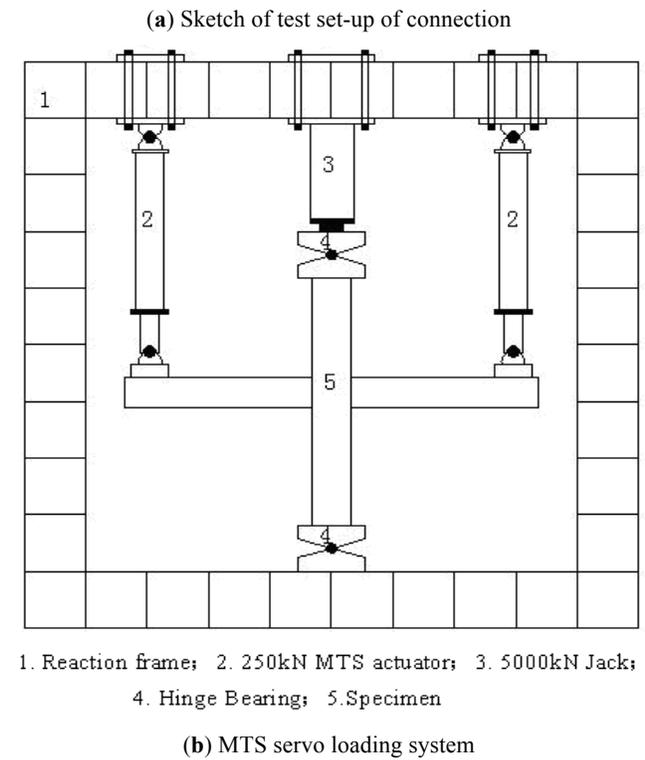


Fig. (3). Distribution of Strain gauges applied to the specimens.



Fig. (2). Test set-up and loading system.

performance of the presented connections. For specimens with axial compression ratio less than or equal to 0.3, the hardening phase displays longer time until serious buckling. Whereas, for specimens with high axial ratio such as, 0.6, the hardening phenomena does not become evident. On the basis of the comparison between specimens, it can be also seen that the increase of ring plate width and the installation of backing plate contribute positively to the hysteretic

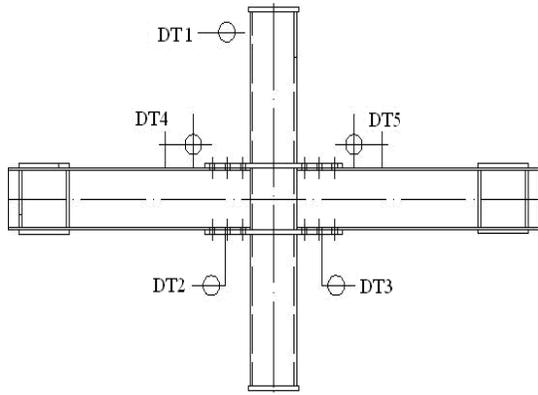


Fig. (4). Distribution of displacement transducers.

performance of the connection specimens. Fig. (6) shows the the skeleton curves of load *versus* displacement relationships. Similarly, it also can be indicated that the increase of ring plate width and the installation of backing plate contribute positively to the deformation capacity of the connection specimens. And the increase of axial compression ratio has negative effects on the bearing capacity and deformation performance of the connection specimens in accordance with what is displayed in the hysteretic curves above. Furthermore, it demonstrates that the specimen with square ring plate presents higher ultimate and failure loads than ones with circular ring plates. Integrated with typical indexes obtained from skeleton curves listed in Table 3 it can be seen that, especially, the specimen with square ring plate presents better bearing capacity than ones with circular ring plates but less deformation performance.

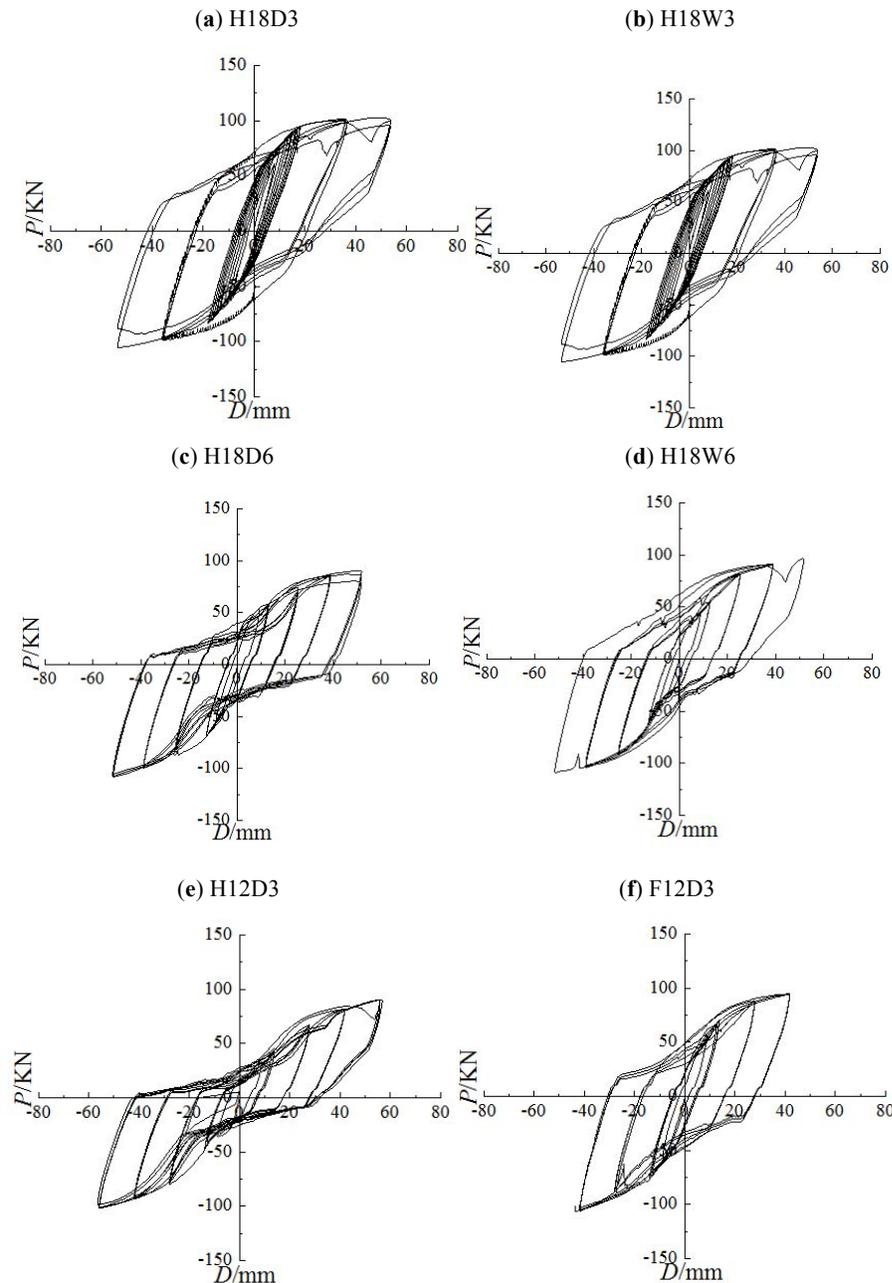


Fig. (5). Comparisons of P- Δ hysteretic curves of connection specimens.

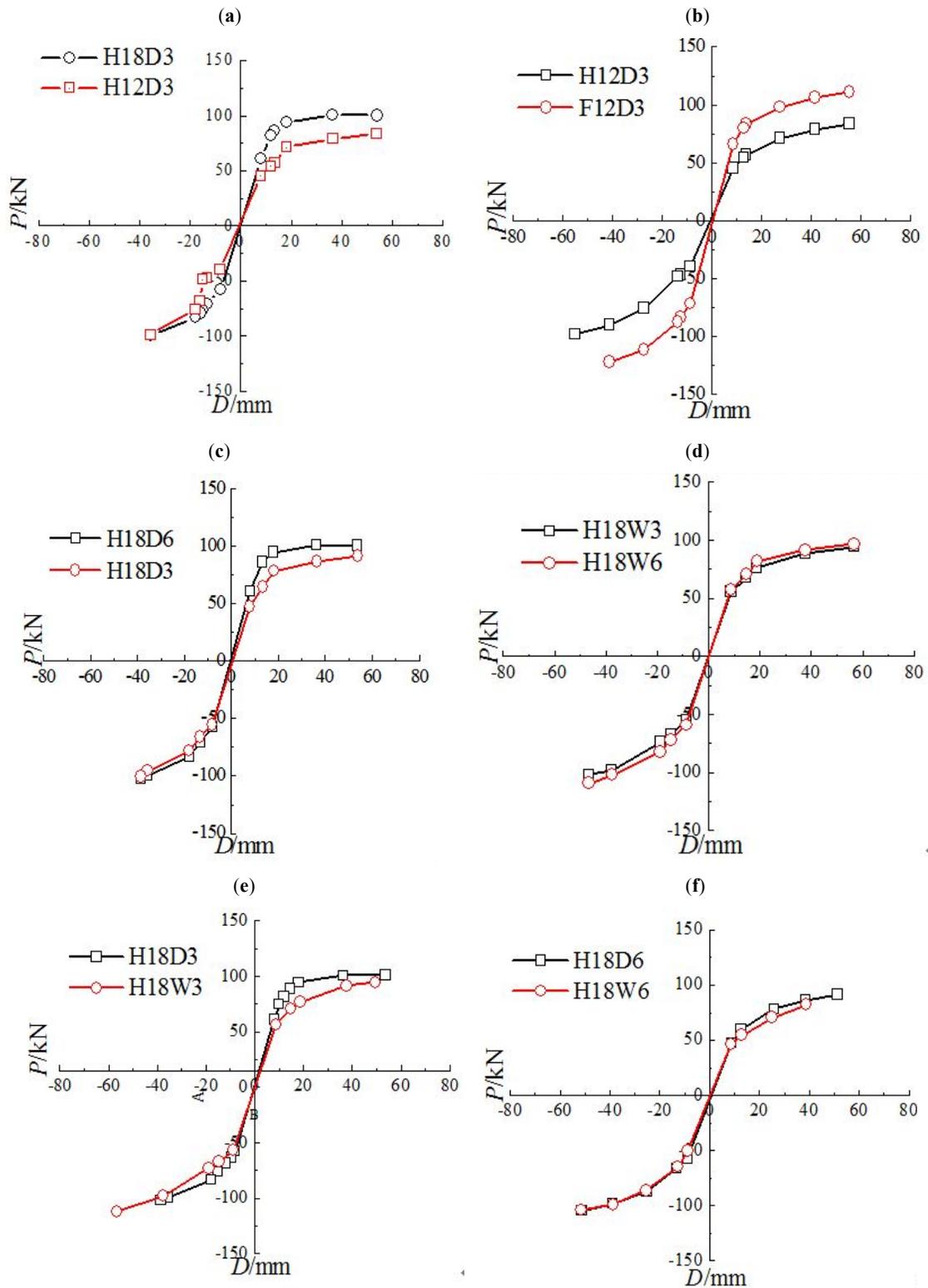


Fig. (6). Comparisons of P-Δ skeleton curves of connection specimens.

3.2. Coefficients of Ductility and Energy Dissipation

In seismic design of structures, ductility coefficients are important indexes to evaluate the performance of structure deformation, as emphasized in Ref. [17]. In the present work, both displacement ductility ratio and rotational ductility coefficient are considered for analyses as listed in

Table 4. Generally, the higher the displacement ductility ratio is the better ductile performance can be. The ductility coefficients obtained from cyclic tests of all connection specimens generally satisfy the needs noted in ‘Specification for design and construction of steel pipe concrete structure (CECS28:90)’. It is described that the increase of width of

Table 3. Typical indexes obtained from skeleton curves of connection specimens.

Connection Specimen	Yield Load P_y (kN)	Displacement Load Δ_y (mm)	Ultimate Load P_{max} (kN)	Ultimate Displacement Δ_{max} (mm)	Failure Load P_u (kN)	Failure Displacement Δ_u (mm)
H18D3	59.48	13.92	115.92	52.96	98.53	58.52
H18W3	56.71	12.77	112.02	56.6	95.22	59.43
H18D6	67.23	14.67	109.13	51.31	92.76	54.52
H18W6	60.32	13.87	107.31	51.31	91.21	55.43
H12D3	57.63	14.76	101.63	55.56	86.38	59.49
F12D3	56.46	13.92	122.47	43.67	104.09	48.62

Table 4. Ductility coefficients of connection specimens.

Connection Specimen	Yield Displacement D_y (mm)	Ultimate Displacement D_u (mm)	Yield Displacement Angel θ_y	Failure Displacement Angel θ_u	Displacement Ductility Ratio μ	Rotational Ductility Coefficient μ_0
H18D3	17.92	58.33	0.0145	0.0477	3.27	3.29
H18W3	18.28	59.43	0.0145	0.0499	3.25	3.26
H18D6	16.98	54.52	0.0118	0.0392	3.24	3.25
H18W6	16.87	54.65	0.0126	0.0425	3.23	3.25
H12D3	14.76	59.49	0.0121	0.04418	3.23	3.24
F12D3	13.92	48.62	0.0119	0.04388	3.21	3.22

ring plate, and the installation of backing plate as well as the shape change of ring plate from square to circle can improve the ductility of connection significantly. Moreover, the ductility of connection specimens decreases with the augment of axial compression ratio.

3.3. Characteristics of Stiffness Degradation

Fig. (7) illustrates the comparisons of stiffness degradation curves of connection specimens. From the comparative curves, it can be found that the degradation tendency is much more defined for the connection specimens with larger size of ring plates and higher axial compression ratio. The backing plates have the similar effect on the degradation tendency of connection specimens. And it coincides with the indications of skeleton curves above, that the specimen with square ring plates displays a more evident degradation tendency than ones with circular ring plates. That is also to say the specimen with square ring plate presents better bearing capacity but lower deformation capacity.

3.4. Characteristics of Strain Distribution and Variation with Load

The strain *versus* load relationships at typical points in connection specimens are shown in Fig. (8). Generally, when load values are more than ± 50 kN, the strains variation, at typical points in connection specimen with load, begin to present different tendencies. And when load values are more than ± 80 kN, plastic strains at typical points in connection specimen occur. Furthermore, when load values are more than ± 100 kN, the strains at typical points in connection

specimen reach the state of hardening plasticity until the specimens' failure. Especially, plastic strains at typical point 2 and point 49 augment with great increments than the strains at the other points. Integrated with Fig. (2), it can also be seen that there are clear stress concentrations in the position of high-strength steel bolt holes and nearby area of beam flange for the given connection specimens.

CONCLUSION

In the present work, a new type of design of cross-shaped connection with CFST column and assembled steel H-beam was investigated based on experimental analyses. For the connections specimens are studied, several conclusions are drawn as follows.

The presented CFST column connection is of convenient for construction and fine deformation performance compared with the CFST column connections by welding technique. It can satisfy the strength needs and can be applied into future construction by further local optimization in detail.

There are clear stress concentrations in the position of high-strength steel bolt holes and nearby area of beam flange for the given connection specimens. Special importance should be given to corresponding constructional techniques and installations for good quality.

The increase of width of ring plate and the shape change of ring plate from square to circle can both improve the ductility and the characteristics of hysteretic behavior of connection significantly. It is emphasized that the specimen with square ring plates is of better deformation performance but lower bearing capacity than the ones with circular ring plates.

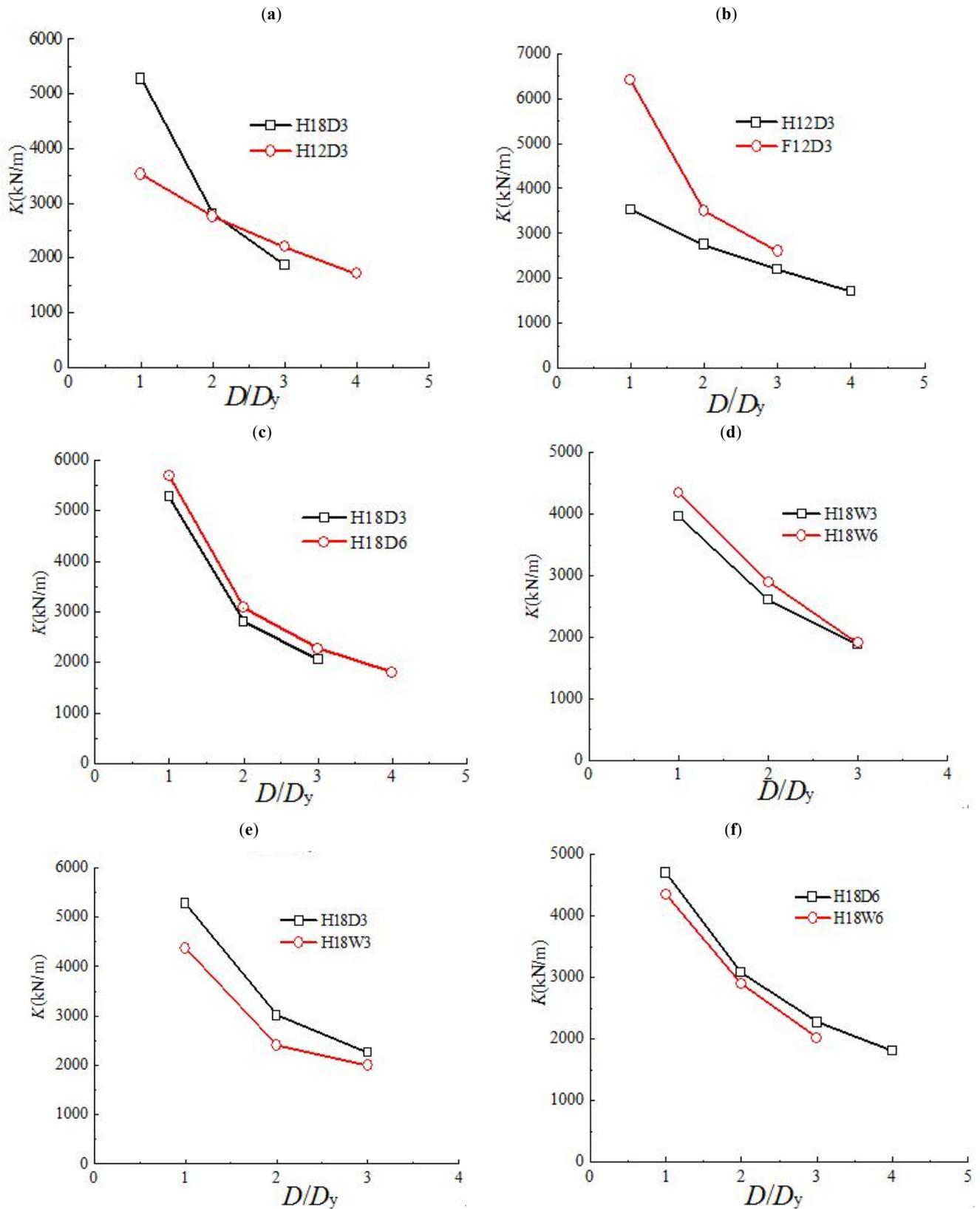


Fig. (7). Comparisons of stiffness degradation curves.

The backing plates have positive effects on the hysteretic characteristics, ductility and energy dissipation of the CFST column connections. It is supposed to be installed in the application for economic considerations and construction

convenience. Conversely, the increase of axial compression ratio contributes negatively to both the bearing capacity and deformation performance of the connection specimens.

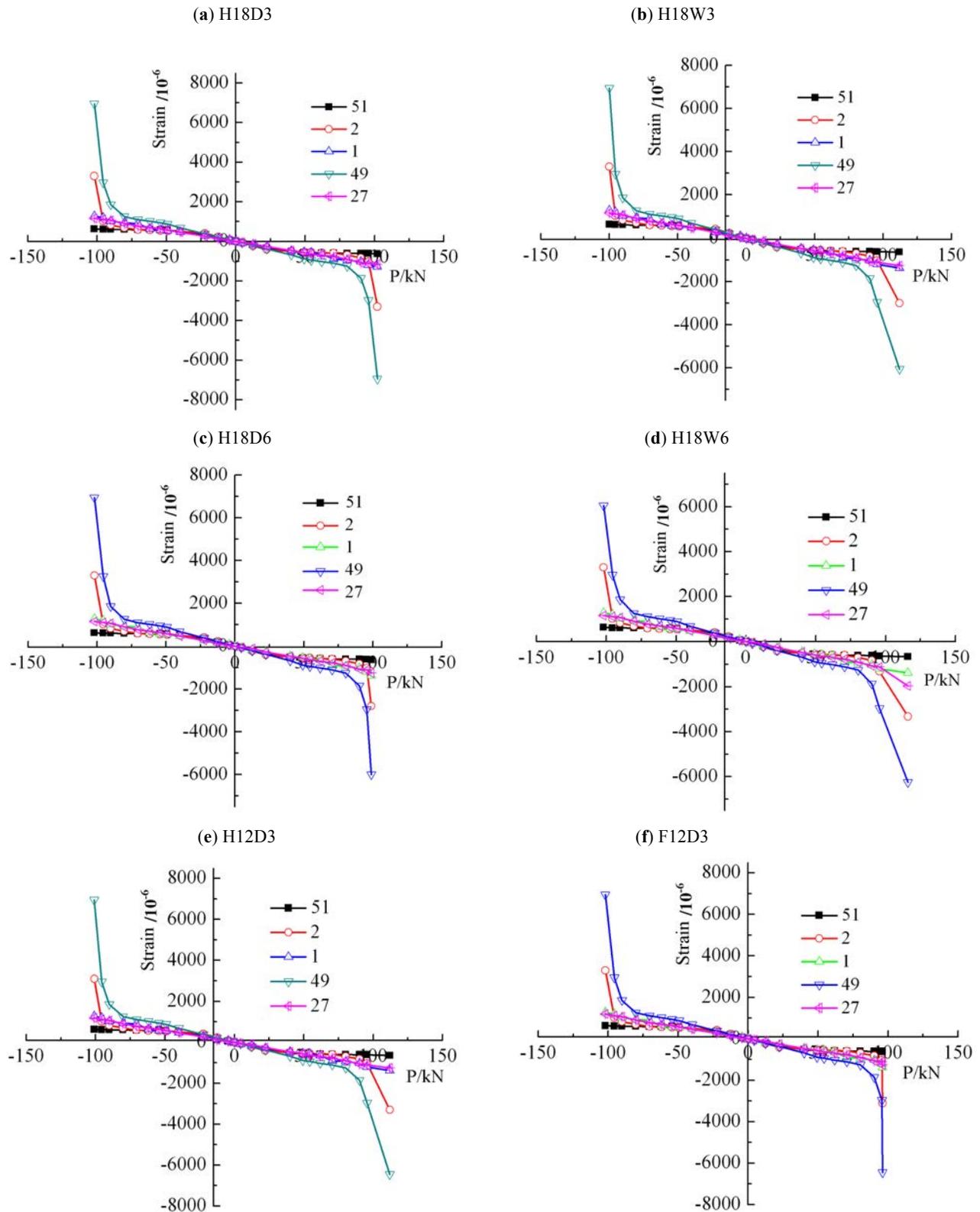


Fig. (8). Strain versus load relationships of typical points in connection specimens.

CONFLICT OF INTEREST

The authors confirm that this article content has no conflict of interest.

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